

Statistical characterization of extreme hydrologic parameters for the peripheral river system of Dhaka city

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Abstract: Selection of appropriate probability distribution function is one of the most important steps of frequency analysis. Due to the existence of large number of distributions, hydrologists follow different methods to select the best one. In this paper, annual maximum, minimum water level and discharge of five peripheral rivers, namely Buriganga, Turag, Tongi, Balu and Lakhya around Dhaka city have been analyzed to compute the basic statistics and fit them with sixty two probability density functions (PDF). Three goodness-of-fit (GoF) statistics, namely Chi-square, Kolmogorov–Smirnov and Anderson Darling were used to rank each of the distribution. Furthermore, ranks obtained from three GoF were used to compute overall rank of all distributions for each hydrologic parameter. The study reveals that, four different distributions were found best fit for four extreme cases. Dagum (4P) and Chi-square (2P) fit best for annual maximum and minimum water level respectively, whereas Cauchy and Johnson SB were found for annual maximum and minimum discharge respectively. Moreover, ranks of frequently used distributions, namely General Extreme Value (GEV), Log-Pearson III (LP3), Log-normal (LN) and Gumbels were compared with the best fit distributions and did not give satisfactory results. The method used in this study would be helpful for flood frequency analysis of other rivers of Bangladesh. This may also be used for evaluation of best fit distribution of river system for other countries as well.

Keywords: Probability Distribution, Rank, Water Level, Discharge, Dhaka City

1. Introduction

Design of different types of hydraulic structures and flood plain zoning, economic evaluation of flood protection projects, etc. require information on flood magnitudes and their frequencies (Rakesh 2005). In addition low flow frequency analysis is also required for assessing water quality, water availability, navigability etc. Moreover climate change associated with global warming got potentiality to change the probability of these events. In particular, highly populated cities surrounded by rivers are vulnerable to flooding and pollution. Urban development and industrialization demand good estimation of both water level and discharge. Thus, it is necessary to explore the water level and discharge extremes around major cities, especially which are in highly populated regions dominated by socio-economic development and prone to pollution and flooding.

Reliable flood frequency distribution selection is one of the major problems faced by the hydrologists. This issue is

very much important as different distributions may produce significantly different estimates for the same return period (Coulson 1991). In order to identify the appropriate probability distribution function a wide range of researches have been conducted previously (e.g., Cunanne 1973; Stedinger 1980; Stedinger et al.1992; Vogel RM 1993; Markiewicz et al. 2006; Mitosek et al. 2006; Laio F et al. 2009; Haddad K. 2010, Rahman A S. et al. 2013). Some of the commonly used distributions for annual maximum flood series include Extreme Value Type 1 (EV1), General Extreme Value (GEV), Extreme Value Type 2 (EV2), Two component Extreme Value, Normal, Log Normal (LN), Pearson Type 3 (P3), Log Pearson Type 3 (LP3), Gamma, Exponential, Weibull, Generalised Pareto and Wakeby (Cunnane 1989; Bobee et al.1993). Furthermore, there is no theoretical basis for the use of a single distribution over others, especially when analyzing a relatively short annual flood series for predicting the magnitude of extreme events (Gumbel 1958). Goodness of fit test is often used to select appropriate probability distribution for frequency analysis. Discrepancies may also arise from the use of different

parameter estimation methods, such as method of moments, maximum likelihood and probability-weighted-moments (Betül Saf 2009). Different goodness-of-fit statistics, such as Chi-square, Kolmogorov–Smirnov and Anderson Darling may also show different ranks for each type of distribution.

A number of studies have been conducted on the rivers of Bangladesh using frequently used distribution functions. Ferdows M (2005) compared three frequently used distributions, namely Log Normal (Two parameters, LN2 and three parameters, LN3); Extreme value Type-I (EVI) or Grumbel and Log-person type-3 (LP3), using annual peak flow of three major rivers (Meghna, Brahmaputra, Ganges-5 stations). In Bangladesh, four distributions are mainly used for at-sit frequency analysis of annual maximum discharge. Gumbel and LN distribution are used by Bangladesh Water Development Board and few departments/ firms respectively, whereas LP3 and GEV were used in National Water Plan and Flood Action Plan respectively (Karim MA and Chowdhury JA. 1995). Karim MA and Chowdhury JA. (1995) suggested the use of GEV for flood frequency analysis of the rivers of Bangladesh. Bari MF. et al.(2002) found that LP3 is more suitable for low flow frequency analysis for the rivers in North-West Bangladesh. So, almost all the researches on low and high flow were conducted using frequently used distributions.

In this study an attempt has been made to identify the best distributions for annual maximum, minimum water level and discharge. The study area is located at one of the most densely populated region in the world. Dhaka, the capital city of Bangladesh is surrounded by a number of rivers, namely Buriganga, Turag, Balu, Lakhya and Tongi. Annual maximum, minimum water level and discharge data of these rivers were used to compute the basic statistics and fit them with sixty two distributions. Three goodness-of-fit (GoF) statistics, namely Chi-square, Kolmogorov–Smirnov and Anderson Darling were used to rank each distribution. Furthermore, median of the ranks obtained from three GoF for each distribution were used to compute overall rank of

all PDFs for each extreme hydrologic parameter, likewise Table 5 shows the ranks for annual maximum water level. Top three distributions for each station and each type of dataset were finally computed. Ranks of frequently used distributions, namely GEV, LP3, LN and Gumbels are also shown to assess their performance for different hydrologic extreme cases.

2. Study Region and Data

2.1. Study Region

Dhaka city is highly populated and influenced by the rapid socio-economic growth. The area of the city is 360 km² with an estimated growth rate of 4.2% per annum that labeled the city as a mega city (Haigh, 2004; Karn and Harada, 2001; Rahman S. and Hossain, F., 2007). Elevation above MSL is 4 m. Buriganga, Turag, Balu, Tongi and Lakhya are the peripheral river system around Dhaka city as shown in Fig 1. The area is greatly affected by the monsoon rainfall leading to flood and water logging. Considerable attention has been paid to understand the occurrence of flood of different return period.

2.2. Data Collection

Yearly maximum, minimum water level for 6 stations and discharge for 3 stations were collected from Bangladesh Water Development Board (BWDB). Location of the gauging stations can be referred to Fig 1. Detailed information of the dataset is given in Table 1. Other than one station on Turag river all the stations are subjected to tidal flow.

3. Methodology

In this research sixty one probability distribution functions (PDF) were used to fit the observed water level and discharge dataset. The list of PDF's used are given in Table 2.

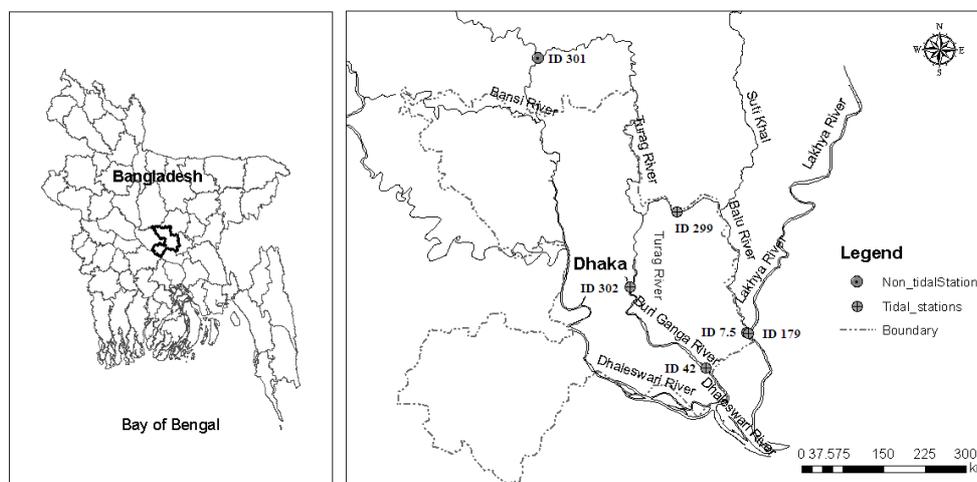


Figure 1. (a) Bangladesh map (b) Peripheral river system of Dhaka city (right).

Table 1. Dataset of water level and discharge station location

Station id	Station	Longitude	Latitude	River	Type	Time interval
301	Kaloikor	90.210	24.082	Turag	Non Tidal WL & Q	1949-2009
42	Dhaka (Mill Barak)	90.445	23.677	Buriganga	Tidal WL	1909-2009
302	Mirpur	90.338	23.783	Turag	Tidal WL	1954-2009
299	Tongi Khal	90.404	23.882	Tongi	Tidal WL	1960-2009
179	Demra	90.505	23.723	Lakhya	Tidal WL & Q	1952-2009
7.5	Demra	90.502	23.723	Balu	Tidal WL & Q	1962-2009

Table 2. Probability density functions used for fitting all hydrological dataset.

Beta	Burr	Burr (4P)	Cauchy	Chi-Squared	Chi-Squared (2P)	Dagum	Dagum (4P)
Erlang	Erlang (3P),	Error	Error Function	Exponential	Exponential (2P),	Fatigue Life	Fatigue Life (3P)
Frechet	Frechet (3P)	Gamma	Gamma (3P)	Gen. Extreme Value	Gen. Gamma	Gen. Gamma (4P)	Gen. Pareto
Gumbel Max	Gumbel Min	Hypersecant	Inv. Gaussian	Inv. Gaussian (3P)	Johnson SB	Kumaraswamy	Laplace
Lognormal	Lognormal (3P)	Nakagami	Normal	Pareto	Pareto 2	Pearson 5	Pearson 5 (3P)
Pearson 6	Pearson6 (4P)	Pert	Power Function	Rayleigh	Rayleigh (2P)	Reciprocal	Rice
Student's t	Triangular	Uniform	Weibull	Weibull(3P)	Johnson SU	Levy	Levy(2P)
Log-Gamma	Log-Logistic	Log-Logistic(3P)	Log-Pearson 3	Log-Normal			

For each station annual maximum, minimum water level and discharge were plotted using all the distributions above (few were found not applicable in some cases). Goodness of fit was performed using Chi-Square, Kolmogorov-Smirnov and Anderson Darling. Ranks according to these three goodness of fit showed a great variation. Median of the ranks obtained from goodness of fit for each PDF was used to rank all the PDFs. A sample example is given in Table 3 showing ranks of maximum water level of Turag river at station 301. PDF's such as General Extreme Value (GEV), Log Pearson Type III, Log Normal and Gumbels which are mostly used for extreme value analysis are shown in Table 3 to highlight their ranking. All the distributions were fitted and ranked using the tool Easyfit. Three goodness of fit method gave separate ranking for each PDF. Median of the ranks obtained from three GoF for each distribution was used to compute the overall rank of all the PDFs. This procedure was followed for maximum, minimum water level and discharge for each station. Finally all these ranks were used to obtain the best fit distributions for each extreme hydrologic parameter. A sample example for station 301 (Turag) is shown in Table 9 (Appendix) which illustrates the best fit distributions obtained following the above procedure. Best fit distributions found from different GoF statistic and median of the ranks for station 301 are shown in Fig 2.

4. Results and Discussion

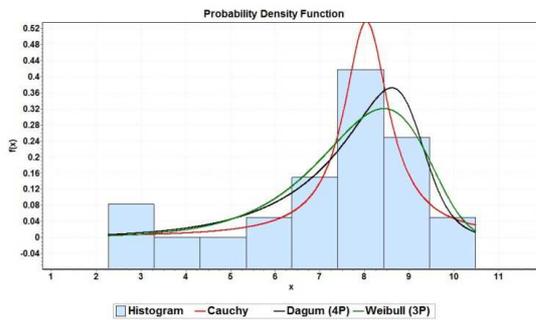
4.1. Basic Statistics

In order to understand the statistical properties of annual maximum and minimum water level and discharge of the peripheral rivers of Dhaka city, basic statistics have been computed (Table 3-4). We have described descriptive statistics such as max, min, range, min, variance, standard

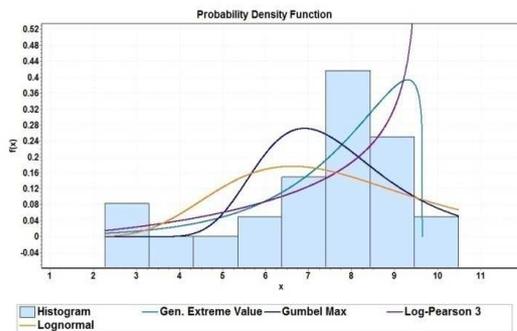
deviation, coefficient of variation, standard error, skewness and excess kurtosis. Considering annual maximum water level (Table 3), station 301 (Turag) gave both highest and lowest WL values of 10.48 and 2.27 mPWD respectively with highest mean of 7.7 mPWD. Maximum and minimum range of 8.21 and 2.55 mPWD were found at station 301 and 179 (Lakhya) respectively. Standard deviation and coefficient of variance were also highest at station 301 with values 1.78 and 0.23 respectively, whereas minimum values were 0.5 and 0.09 respectively at station 179. Other than station 179 all the stations showed negative skewness. In context of excess kurtosis station 299(Tongi) gave highest value of 4.01 and the lowest value 0.69 was found at station 179. Similarly considering annual minimum water level, station 301(Turag) is having highest range 2.42 mPWD (2.91-0.49) and standard deviation 0.56. Whereas station 301 (Lakhya) was found to have lowest range 0.94 mPWD and st. deviation 0.16. Highest and lowest coefficient of variation were found 0.34 and 0.18 at station 301 and 7.5 (Balu) respectively. Stations other than station 301 and 7.5 possess positive skewness. With regard to excess kurtosis station 301 has a negative value of -0.76, whereas maximum value of 3.64 is associated at station 302 (Turag).

Moreover considering annual maximum discharge of all three stations (Table 4), station 179 gives the highest and mean discharge 2610 m³/s and 1947.3 m³/s respectively with minimum standard deviation, coefficient of variance and skewness of 378.44, 0.19 and -1.35 respectively. Maximum standard deviation and coefficient of variation were found 462.94 and 0.91 at station 301 and 7.5 respectively. Excess kurtosis of 13.53 at station 7.5 was quite higher than the minimum value 2.58 found at station 301. Similarly considering annual minimum discharge of all the stations, station 179 gives highest mean and standard deviation of 647.24 m³/s and 316.94 with negative skewness and kurtosis of -0.02 and -0.81 respectively.

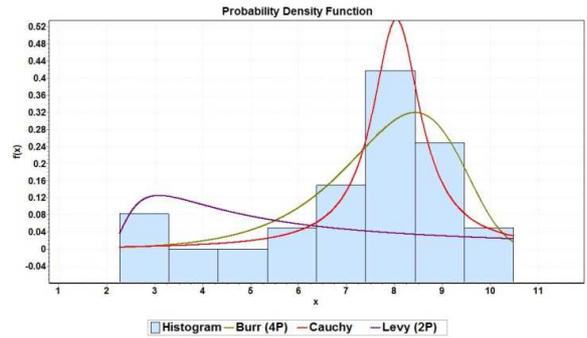
Whereas lowest mean and standard deviation were computed at station 301, though it possess highest coefficient of variation of 1.04 and excess kurtosis 14.77. Considering all discharge dataset it was found that discharge variation is highest at station 301 (2200-0.42 m³/s), standard deviation also varied greatly (462.94 to 3.26) at this station. Station 179 has a tendency to show negative skewness which is opposite to others. So, considering all the annual water level dataset highest variation (10.48-0.49) and standard deviation (1.78, 0.56) are associated with station 301 which is non tidal in behavior. Most of the stations showed negative skewness while annual maximum WL considered, whereas the scenario was vice versa considering annual minimum WL. Other than one case (Annual minimum WL, station 301 = -0.76) all the stations showed positive excess kurtosis.



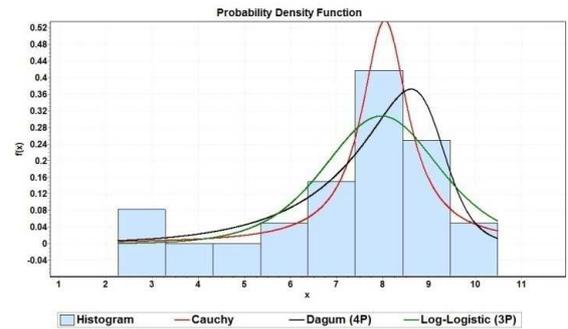
a)



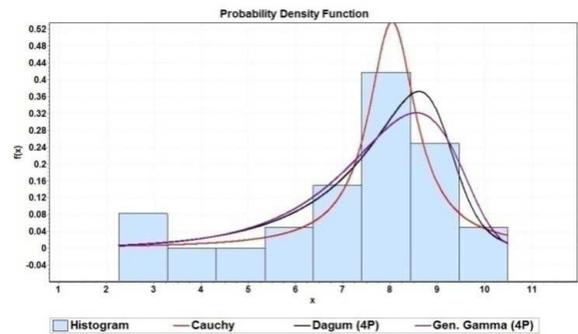
b)



c)



d)



e)

Figure 2. Probability density function plotting of station 301 for annual maximum WL a) Best 3 based on the median of ranks obtained from Goodness-of-fits b) Frequently used distributions c) Best 3 according to Kolmogorov Smirnov d) Best 3 according to Anderson Darling e) Best 3 according to Chi-Squared

Table 3. Descriptive statistics of annual water level (mPWD) for individual stations

Station Id	River	Max	Min	N	Range	Mean	Variance	Std. Dev	Coeff. of variation	Std. error	Skewness	Excess Kartosis
Annual Max. Water Level (mPWD)												
301	Turag	10.48	2.27	60	8.21	7.7	3.18	1.78	0.23	0.23	-1.8	3.38
42	Buriganga	7.58	3.2	94	4.38	5.77	0.38	0.62	0.11	0.06	-0.37	3.12
302	Turag	8.35	2.71	54	5.64	6.18	0.9	0.95	0.15	0.13	-0.63	2.57
299	Tongi	7.84	2.71	49	5.13	5.99	0.88	0.94	0.16	0.13	-1.34	4.01
179	Lakhya	7.11	4.56	48	2.55	5.84	0.25	0.50	0.09	0.07	0.39	0.69
7.5	Balu	7.09	2.57	46	4.52	5.82	0.47	0.69	0.12	0.10	-2.19	10.47
Annual Min. Water Level (mPWD)												
301	Turag	2.91	0.49	60	2.42	1.66	0.31	0.56	0.34	0.07	-0.17	-0.76
42	Buriganga	1.23	0.24	95	0.99	0.66	0.03	0.16	0.25	0.02	0.48	1.82
302	Turag	1.71	0.12	56	1.59	0.80	0.06	0.24	0.29	0.03	0.64	3.64
299	Tongi	1.52	0.53	50	0.99	0.94	0.04	0.20	0.21	0.03	0.86	1.15
179	Lakhya	1.42	0.48	49	0.94	0.84	0.03	0.16	0.20	0.02	0.98	2.83
7.5	Balu	1.4	0.34	48	1.06	0.93	0.03	0.17	0.18	0.02	-0.29	2.96

Table 4. Descriptive statistics of discharge(m³/s) for individual stations

Station Id	River	Max	Min	N	Range	Mean	Variance	Std. Dev	Coeff. of variation	Std. error	Skewness	Excess Kurtosis
Annual Max. Discharge (m ³ /s)												
301	Turag	2200	4	32	2196	686.11	214320	462.94	0.67	81.84	0.91	2.58
179	Lakhya	2610	774	26	1935.8	1947.3	143210	378.44	0.19	74.22	-1.35	4.3
7.5	Balu	2077.3	140	21	1937.3	451.47	167850	409.69	0.91	89.4	3.52	13.53
Annual Min. Discharge (m ³ /s)												
301	Turag	18.2	0.42	32	17.78	3.14	10.64	3.26	1.04	0.58	3.38	14.77
179	Lakhya	1300	106	26	1194	647.24	100450	316.94	0.49	62.16	-0.02	-0.81
7.5	Balu	288.52	16.1	21	272.42	73.33	4214.9	64.92	0.89	14.17	2.13	5.38

Table 5. Best fit distributions and ranks of GEV, Log Pearson III, Log Normal and Gumbels for annual maximum water level.

St. id	Best 3 distributions			Ranks of mostly used PDF			
	#1	#2	#3	GEV	Log Pearson III	Log Normal	Gumbels
Annual Max. water level							
301	Cauchy	Dagum (4P)	Weibull (3P)	25	31	41	34
42	Burr (4P)	Hypersecant	Dagum	11	46	29	28
302	Hypersecant	Log-logistic	Laplace	28	47	29	35
299	Log logistic (3P)	Laplace	Dagum(4P)	27	50	35	30
179	Dagum	Burr	Log-Logistic	25	11	14	44
7.5	Cauchy	Dagum(4P)	Log logistic (3P)	29	52	34	28
All	Dagum (4P)	Log-Logistic (3P)	Burr (4P)	24	30	46	31

Table 6. Best fit distributions and ranks of GEV, Log Pearson III, Log Normal and Gumbels for annual minimum water level.

St. id	Best 3 distributions			Ranks of mostly used PDF			
	#1	#2	#3	GEV	Log Pearson III	Log Normal	Gumbels
Annual Min. water level							
301	Dagum(4P)	Gen. Gamma	Error	10	29	16	28
42	Laplace	Erlang	Hypersecant	14	33	34	41
302	Dagum(4P)	Log-logistic	Burr	8	7	14	34
299	Chi-squared	Burr (4P)	Chi-squared(2P)	17	11	19	43
179	Cauchy	Chi-squared(2P)	Chi-squared	24	21	17	44
7.5	Chi-squared(2P)	Log logistic (3P)	Logistics	13	48	34	32
All	Chi-Squared (2P)	Burr (4P)	Chi-Squared	10	28	16	42

Table 7. Best fit distributions and ranks of GEV, Log Pearson III, Log Normal and Gumbels for annual maximum discharge.

St. id	Best 3 distributions			Ranks of mostly used PDF			
	#1	#2	#3	GEV	Log Pearson III	Log Normal	Gumbels
Max. Discharge							
301	Error	Hypersecant	Laplace	10	27	45	16
179	Cauchy	Log-logistic(3P)	Laplace	20	40	23	29
7.5	Cauchy	Burr	Log-logistic(3P)	19	31	12	19
All	Cauchy	Laplace	Log-Logistic (3P)	17	36	25	18

Table 8. Best fit distributions and ranks of GEV, Log Pearson III, Log Normal and Gumbels for annual minimum discharge.

St. id	Best 3 distributions			Ranks of mostly used PDF			
	#1	#2	#3	GEV	Log Pearson III	Log Normal	Gumbels
Min. Discharge							
301	Fatigue Life	Logistic	Log Normal (3P)	8	7	32	47
179	Dagum	Error	Johnson AB	4	10	40	19
7.5	Gen. Pareto	Johnson AB	Fatigue Life (3P)	4	12	2	52
All	Johnson SB	Gen. Extreme Value	Inv. Gaussian (3P)	2	8	35	50

4.2. Ranking of Probability Distribution Function

All the PDF were ranked for both water level and discharge at each station. Finally all those ranks were used to determine the ranking of PDF for similar type of dataset, such as max. wl, min. wl, max. Q and min. Q. Table 5-8 describes best three distributions for each type of dataset, ranks of mostly used distributions are also added in those tables.

Best fit distributions for annual maximum water level are listed in Table 5. It reveals great combination of various types of distributions. Considering all dataset Dagum(4P), Log-logistic(3P) and Burr (4P) were found the best three. On the other hand ranks of frequently used distributions GEV, LP3, LN and Gumbels were 24, 30, 46 and 31 respectively. Similarly, ranks of distributions for minimum WL are listed in Table 6. Frequently used distributions fell behind in this case too. The ranks obtained are 10, 28, 16 and 42 respectively. Whereas the best fit distributions are Chi-squared (2P), Burr (4P) and Chi-Squared.

Similar to water level, discharge dataset were also fitted to different distributions (Table 7-8). In case of annual maximum discharge various distributions were found best fit for different stations. Frequently used distributions didn't show good performance in this case. Ranks of frequently used distributions GEV, LP3, LN and Gumbels were 17, 36, 25 and 18 respectively. Best fit distributions were Cauchy, Laplace and Log-logistic (3P). Whereas, considering annual minimum discharge GEV (Generalized Extreme Value) gave a good result in combined ranking. But separately each station gave different best fit distributions. Best fit distributions for annual minimum water level were Johnson SB, GEV and Inv. Gaussian (3P).

Appendix

Table 9. Ranking of PDF for maximum water level of Turag river at Kaloikor station (st. id 301)

Distribution	Kolmogorov Smirnov Ranking of PDF for maximum water level of Turag river at Kaloikor station (st. id 301)		Anderson Darling		Chi-Squared		Median	
	Statistic	Rank	Statistic	Rank	Statistic	Rank	Value	Rank
Beta	0.15	10	1.69	7	11.00	15	10	9
Burr	0.16	12	2.78	13	11.09	16	13	13
Burr (4P)	0.11	5	1.20	6	4.12	3	5	4
Cauchy	0.09	1	0.61	1	0.79	1	1	1
Chi-Squared	0.42	51	13.42	47	85.33	53	51	53
Chi-Squared (2P)	0.37	50	10.98	43	70.84	51	50	51
Dagum	0.16	11	1.87	9	13.21	19	11	10
Dagum (4P)	0.10	3	0.97	2	10.52	14	3	2
Erlang	0.29	42	8.43	37	23.58	37	37	36
Erlang (3P)	0.22	28	4.39	21	16.77	31	28	27
Error	0.19	18	2.73	12	8.36	10	12	12
Error Function	0.92	59	554.77	60	932.97	54	59	59
Exponential	0.47	53	17.66	51	22.18	34	51	53
Exponential (2P)	0.43	52	14.76	49	8.86	12	49	49

5. Conclusion

In this study annual maximum, minimum water level and discharge of the peripheral river system of Dhaka city have been analyzed to identify the best fit distribution among sixty two distributions. Three goodness-of-fit statistics (GoF), namely Chi-square, Kolmogorov-Smirnov and Anderson Darling were used to rank all distributions median of the ranks obtained from three GoF for each distribution were used to compute overall rank of all PDFs for each extreme hydrologic parameter. The study reveals that, four different distributions were found best fit for four extreme hydrologic cases. Dagum (4P) and Chi-square (2P) fit best for annual maximum and minimum water level respectively, whereas Cauchy and Johnson SB were found for annual maximum and minimum discharge respectively. The ranks of frequently used distributions GEV, LP3, LN and Gumbels were not satisfactory for almost all the hydrologic parameters.

Recommendation

The method used in this study would be helpful for flood frequency analysis of other rivers of Bangladesh. This may also be used for evaluation of best fit distribution for other countries as well.

Acknowledgements

Authors are gratefully acknowledging the cooperation rendered by Bangladesh Water Development Board (BWDB) for providing the necessary data.

Distribution	Kolmogorov Smirnov Ranking of PDF for maximum water level of Turag river at Kaloikor station (st. id 301)		Anderson Darling		Chi-Squared		Median	
	Statistic	Rank	Statistic	Rank	Statistic	Rank	Value	Rank
Fatigue Life	0.31	44	8.52	38	37.72	43	43	42
Fatigue Life (3P)	0.21	25	3.96	19	16.46	26	25	22
Frechet	0.37	49	12.13	45	67.57	50	49	49
Frechet (3P)	0.28	40	7.57	34	25.91	38	38	38
Gamma	0.23	30	6.18	29	13.27	20	29	28
Gamma (3P)	0.21	26	4.40	22	17.30	32	26	24
Gen. Extreme Value	0.13	8	12.53	46	N/A		27	25
Gen. Gamma	0.26	38	6.63	31	28.76	40	38	38
Gen. Gamma (4P)	0.12	7	1.12	3	6.06	7	7	7
Gen. Pareto	0.18	16	33.68	58	N/A		37	36
Gumbel Max	0.26	36	11.84	44	16.73	30	36	34
Gumbel Min	0.14	9	1.71	8	8.17	8	8	8
Hypersecant	0.18	15	2.91	14	11.65	17	15	15
Inv. Gaussian	0.20	23	7.23	33	15.92	22	23	19
Inv. Gaussian (3P)	0.20	22	3.88	17	16.45	25	22	18
Johnson SB	0.18	14	19.27	53	N/A		33.5	33
Kumaraswamy	0.12	6	1.20	5	4.12	5	5	4
Laplace	0.19	19	2.73	11	8.36	11	11	10
Levy	0.58	57	22.39	55	36.62	42	55	57
Levy (2P)	0.49	56	16.29	50	1.48	2	50	51
Log-Gamma	0.31	45	9.64	40	38.01	44	44	45
Log-Logistic	0.30	43	8.20	36	47.62	47	43	42
Log-Logistic (3P)	0.10	2	1.97	10	5.54	6	6	6
Log-Pearson 3	0.18	17	14.32	48	N/A		32.5	31
Logistic	0.19	20	3.19	16	16.54	27	20	16
Lognormal	0.29	41	7.86	35	35.34	41	41	41
Lognormal (3P)	0.21	27	4.25	20	16.71	28	27	25
Nakagami	0.17	13	5.07	26	10.46	13	13	13
Normal	0.21	24	3.94	18	16.45	24	24	20
Pareto	0.49	55	19.40	54	8.25	9	54	56
Pareto 2	0.48	54	18.33	52	22.45	35	52	55
Pearson 5	0.32	46	9.51	39	50.62	48	46	46
Pearson 5 (3P)	0.24	33	5.02	25	14.06	21	25	22
Pearson 6	0.27	39	7.15	32	41.17	45	39	40
Pearson 6 (4P)	0.25	34	5.25	28	20.05	33	33	32
Pert	0.19	21	3.01	15	16.25	23	21	17
Power Function	0.22	29	4.74	24	12.12	18	24	20
Rayleigh	0.32	47	9.95	42	78.45	52	47	48
Rayleigh (2P)	0.35	48	9.64	41	47.13	46	46	46
Reciprocal	0.58	58	32.26	57	27.50	39	57	58
Rice	0.25	35	5.15	27	16.73	29	29	28
Student's t	0.92	60	208.24	59	1199.10	55	59	59
Triangular	0.26	37	4.69	23	22.68	36	36	34
Uniform	0.24	31	22.66	56	N/A		43.5	44
Weibull	0.24	32	6.53	30	55.19	49	32	30
Weibull (3P)	0.11	4	1.19	4	4.12	4	4	3
Johnson SU	No fit	-	-	-	-	-	-	-

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