



Wooden I-Beams Made of Round Timber with a Core Rot

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Abstract: Round timber with core rot was not used in the production of sawn timber for load-bearing building structures. The purpose of this work is the researches of a possibility of receiving wooden beams from round timber with a core rot. The wooden I-beam made of angular elements of the received their round forest products with decay was chosen as the object being studied. The design scheme for determining strength and deflections of the beam is a statically determinate hinged beam, loaded with a uniformly distributed load. The design spacing of the structure is assumed to be 6 meters. Under the action of a load equal to 1.5 kN/m, the beam is exposed to stress-strain state of transverse bending. A method for manufacturing I-beams from corner elements obtained as a result of cutting round logs with core rot is proposed. It has established that the calculated strength characteristics of I-beams produced according to the proposed technology may resist operational loads. Rated and tangential stresses arising from application of loads do not exceed the permissible values. Deflection of a beam does not exceed allowable deflection values. The use of round timber with core rot increases resources of wood for construction.

Keywords: I-Beam, Low-Grade Forest Products, Angle Bar Elements, Strength Characteristics of Wood

1. Introduction

I-beams from wood are now widely used in the construction of buildings and structures. They are used for installation of ceilings, rafters, installation of floors and strapping on the foundation. The cross-sectional shape of the beam ensures a good performance of the structure in terms of bending under operating conditions. In terms of their performance, they are not inferior to metal and reinforced concrete structures, and they have less weight. Beams withstand design loads, both at small and large spans (up to 12 m). However, modern construction experience shows the expediency of using I-beams for spans from 2 to 6 m [1].

The advantages of I-beams refer to light weight, high safety margin, ease of transportation and installation and cost-effectiveness. Light weight, about 50 kN/running meter, allows you working without heavy equipment. The high speed of installation and ease of assembly increase adaptability of erection of buildings and structures. After treatment with special compounds, the wood of the beams has the required fire resistance, resistance to decay and insect damage. There is a great demand for such beams in seismically dangerous areas of construction. The ecological properties of wood beams are not questioned. The retail price

of I-beams is not high. For example, cost of a beam with a height of 200 mm and dimensions of the cross section of the belts of 42 x 85, is about 300 rubles per running meter.

Calibrated block of glued timber is used for production of shelves in the beams manufactured using the technology proposed in Canada, and the walls are made of oriented structural boards OSB-3 and OSB-4. In Russian conditions, most walls are made from plywood and rarely from LVL.

A large number of works have been devoted to the study of the features of I-beams. In particular, the publication of [2] presents the results of tests of an I-beam with a wall from OSB. In the paper [3], strength and wood elasticity of composite beams with a wall from an oriented structural board are considered. In the studies performed [4-17], the issues of influence of anisotropy of the wood properties and various methods of fastening and strengthening on the stress-strain state of the structure have been considered.

Low-quality round timber is timber that is not allowed for processing for the production of saw-timbers used in construction, furniture production and other areas of production where a certain level of quality is required. Up to 85% of product range from coniferous wood fall into the category of low- quality due to pressed rot. Rot damages the core wood and extends to one or both ends of the assortment. Low-quality timber is used for production of chips in

chemical-mechanical processing of wood raw materials or wood. Only additional treatment and processing of low-grade wood allow using it for production of short-length sawn timber and rough furniture blanks.

Studies have established that the change in diameter of the stump rot along the length of the tree trunk occurs in accordance with law of constancy of relative growth [18]. In accordance with this law, the diameter of the stump rot in an arbitrary section of the tree trunk is determined by the formula:

$$d_r = d_{r0} - a l_r^b, \quad (1)$$

where d_r - diameter of stump rot in an arbitrary cross-section, m; d_{r0} - diameter of rot at the butt-end of round product range, m; l_r - length affected with stump rot, m; a, b - constants of initial condition and balance, respectively.

The main task of sawing low-quality round timber is to obtain the maximum quantity of sawn timber with the required quality and free from any rot. The studies [19] have suggested a log cutting plan on the generator line, taking into account the structure of wood and its physical and mechanical properties.

Recently, methods for cutting low-quality round timber with core rot in order to produce sawn timber of a large cross-section have been proposed. For example, it has been proposed to fix the timber with regard to healthy peripheral region and divide the timber into sectors in the longitudinal direction. For the maximum quantity of useful wood round forest products and a bar sawed parallel to one or two edges. Then, the end faces of the central blanks are mated and fastened together. The drawbacks of this method of the cutting method refer to the need to achieve high precision in the manufacture of mating blanks and a low volumetric yield of large-diameter sawn goods.

Another method involves dividing the timber into parts in the longitudinal direction to produce three-edged cants and sectors. Then, the shape and parameters of the core layer are determined and the low-quality core layer is removed. Further processing includes forming a surface treatment of the same shape and taper in length, rotating adjacent portions with regard to each other by 180° in the horizontal plane, mating surfaces being processed and bonding them together. The disadvantage of this method refers to considerable labor costs in obtaining the timber and the need for its refinement for obtaining structural timber.

The following method for cutting logs with core rot is also known. Preliminary longitudinal sawing of short-length round timber is ensured. One or more longitudinal cuts is made. From the parts of the timber obtained after sawing, the core layer is removed, corresponding to the dimensions of the formed voids in these parts, the billets are made from wood. From the obtained parts of timber and joint blanks after drying, long timber is formed, while the ends of the contact of wooded parts along the length of the timber being formed are displaced with regard to each other [20]. However, the

proposed methods do not provide for the production of structural timber for construction.

The purpose of this work is the research of a possibility of application of I-beams from wooden angular elements as the bearing building constructions. For achievement of the goal it is necessary to solve the following problems: to develop a method of production of I-beams from round forest products with decay and to check a possibility of application of wooden I-beams as the bearing building constructions.

2. Methods

The wooden I-beam made of angular elements of the received their round forest products with decay was chosen as the object being studied. Production is made according to the following scheme. The bark is removed in the product range with core rot with an outlet to both ends. Cutting is ensured according to the timber scheme. On the first pass two-cant beams with core rot and side unedged thin lumber are produced. On the second pass two bars with core rot, as well as side unedged sawn timber are produced. Then the bars are subjected to chamber drying by soft modes and their moisture content is adjusted to $14 \pm 2^\circ\text{C}$. Rotting is removed by milling. The bars are sorted in such a way that the quality of the wood corresponds to grade 2 according to state standard. This quality corresponds to the strength class K24.

To cutting pine round timber with core rot with a diameter in the apex of 30 cm and a length of 6.5 m was chosen. The run was taken equal to 0.8. Thus, the diameter of the assortment in the lump is 35.2 cm. The dependence of the rotting diameter along the length of the trunk in spruce timber is defined by the formula [13].

$$d_r = 0.44 - 0.0576 l_r^{1.0346} \quad (2)$$

where d_r - rotting diameter, cm; l_r - distance from the face end, cm.

On the butt-end of timber, the rotting diameter is 15.6 cm.

Theoretical cutting of these round timber is executed. The maximum theoretical yield of sawn timber is obtained by sawing a two-cant timber at the first pass with a thickness equal to 0.707 vertex diameter. On the second pass, the total width of the central timber should also be close to this size. Proceeding from these positions at cutting of product range with vertex diameter of 30 cm on the first pass a two-cant bar in the thickness of 217 mm, width of a plate of 108 mm with a rot with sizes of 156 mm on a lump of a bar is obtained. Also, side-edged boards of 25 mm in thickness, equal in length or less than the length of the assortment (Figure 1) are obtained. The width of the cut is assumed equal to 3 mm. On the second pass from the central part of the two-cant beam we obtain two bars with cross-sectional dimensions of 102×217 mm. Out of the peripheral part of the two-cant beam, side boards with a thickness of 25 mm are obtained, with length equal to or less than the length of the range. The bars are placed in drying piles and dried to humidity of $14 \pm 2^\circ\text{C}$. Then the bars are milled lengthwise and cut into two bars of

angular cross-section of 98×103 mm with wall thickness of 21 and 27 mm (Figure 2).

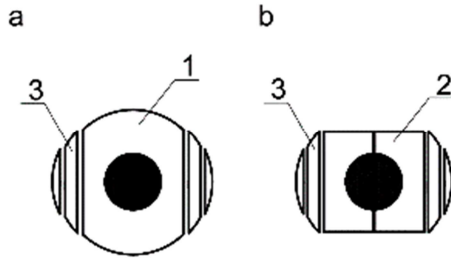


Figure 1. Scheme of cutting round timber: a - sawing of two-cant timber and side unedged lumber; b - sawing of a double-cant beam.

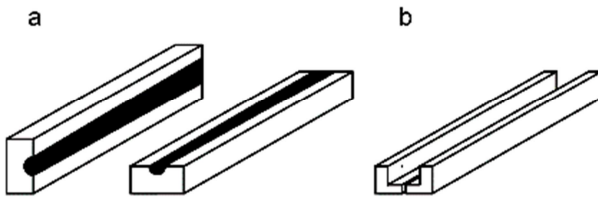


Figure 2. Milling and cutting of bars with core rot: a - bar with core rot; b - angle bars.

The yield of the bars of the angular cross-section from the round assortment is 20%. The yield of side unedged boards is 22.5%. The overall useful yield is 42.5%.

The bars are glued together in pairs and the work pieces obtained in the form of channels are glued together to form an I-beam (Figure 3). The beam has a height of 207 mm, a width of 196 mm, a belt thickness of 27 mm and a wall thickness of 43 mm (Figure 4).

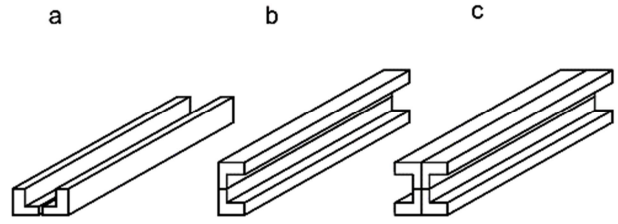


Figure 3. Bonding of the angular profile bars into the I-beam: a - bars of angular profile; b - blocks glued to the channel; c - beam joined from channels.

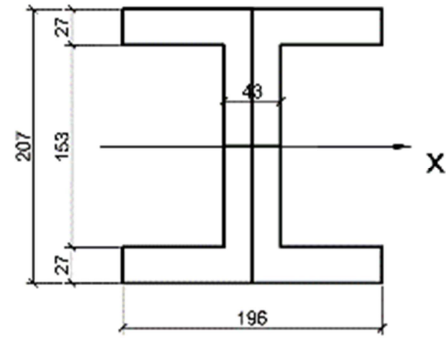


Figure 4. Cross section of the beam.

3. Results and Discussion

The design scheme for determining strength and deflections of the beam is a statically determinate hinged beam, loaded with a uniformly distributed load. The design spacing of the structure is assumed to be 6 meters. Under the action of a load equal to 1.5 kN/m, the beam is exposed to stress-strain state of transverse bending. Strength characteristics of wood in two directions, adopted in accordance with SR 64.13330.2017 "Wooden structures" are given in the table. 1. Coefficients for determination of settlement resistance are specified in table. 2.

Table 1. Characteristics of wood.

Parameters	Designation	Units	2 grade/class K24
Calculated resistance to bending	R_m^A	MPa	22.5
Calculated resistance to splitting across wood fibers	$R_{90,d}^A$	MPa	2.4
Average modulus of flexure for calculation of bending	$E_{0,mean}$	GPa	11.0

Table 2. Coefficients for calculation.

Parameters	Designation	Quantity
Coefficient of long action of loading for strength characteristics of wood	m_l	0.66
Coefficient of long action of loading for elastic characteristics of wood	$m_{l,E}$	1.0
Coefficient of breed of wood	m_b	1.0
Coefficient of service conditions of designs	m_c	1.0
Coefficient of temperature	m_t	1.0

3.1. Rated Stress

Rate stresses in the beam is determined by a formula:

$$\sigma = \frac{M}{W} \leq R_m^d, \quad (3)$$

where M - maximum bending moment, kN·m; W - moment

of resistance of lower stretched belt, m_3 ; R_m^d - tolerable calculated resistance to bending for 2 grade.

Maximum bending moment determined by a formula:

$$M = \frac{q \cdot l^2}{8}, \text{ kN} \cdot \text{m}, \quad (4)$$

where q - calculated load, kN/m; l - beam span, m.

Tolerable calculated bending resistance is determined by formula:

$$R_m^d = R_m^A \cdot m_l \cdot \prod m_i, \quad (5)$$

where R_m^A - calculated bending resistance of wood, MPa (table 1); m_l - coefficient of long action of loading for

strength characteristics of wood (table 2); $\prod m_i$ - coefficients of operating conditions of a design m_b , m_c , m_t (table 2). Results of calculations of the normal tension arising in a beam and the allowed settlement resistance at a bend are given in table 3 and table 4.

Table 3. Rated stress arising in a beam when bending.

Parameters	q^d , kN/m	l , m	M , kN/m	W , m ³	Results of calculation, MPa
Rated stress arising in a beam when bending	1.5	6.0	6.75	0.0011	6.14

Table 4. Tolerable calculated resistance to bending.

Parameters	R_m^A , MPa	m_l	m_b	m_c	Results of calculation, MPa
Tolerable calculated resistance to bending	22.5	0.66	1.0	1.0	14.85

Comparison of values of normal tension in a beam and the allowed settlement resistance at a bend is given in figure 5. The analysis shows that the size of normal tension in a beam, arising at a bend under loading, is 41% of size of the allowed settlement resistance.

3.2. Beam Deflection

Deflection is calculated by a formula:

$$f = \frac{5q^k \cdot l^4}{384 \cdot E^{II} \cdot I}, \quad (6)$$

where q^k - rated load, kN/m; l - beam span, m; E^{II} - calculated modulus of elasticity of wood in calculation with regard to extreme conditions of 2nd group, GPa; I - moment

of inertia of the beam cross-section, m⁴.

Rated load equals 1.25 kN/m. Beam span is 6 m. Calculated modulus of flexure of wood in calculations with regard to extreme conditions of 2nd group is calculated by formula:

$$E^{II} = E_{0, \text{mean}} \cdot m_l \cdot \prod m_i \quad (7)$$

where $E_{0, \text{mean}}$ - average modulus of flexure, GPa (table 1);

$m_{l, E}$ - index for elastic characteristics (table 2); $\prod m_i$ - product of indices of operating conditions m_b , m_c , m_t (table 2).

Table 5. Rated deflection of a beam.

Parameters	q^k , kN/m	l , m	$E_{0, \text{mean}}$, GPa	$m_{l, E}$	m_b	m_c	m_t	E^{II} , GPa	I , m ⁴	Results, mm
Rated deflection of a beam	1.25	6.0	11.0	1.0	1.0	1.0	1.0	11.0	$99.2 \cdot 10^{-6}$	19.4
Tolerable calculated deflection of a beam	-	-	-	-	-	-	-	-	-	$l / 250$ 24.0

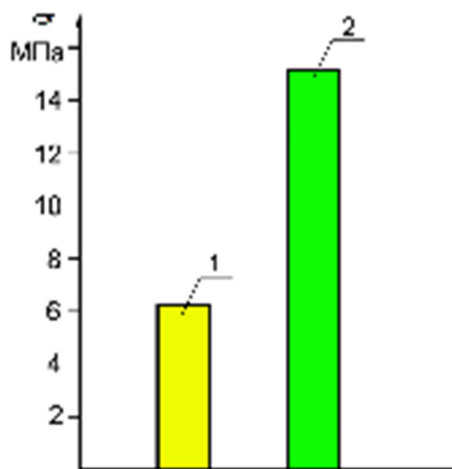


Figure 5. Comparison of rated stress with tolerable calculated resistance to bending: 1 – the rated stress of in a beam when bending; 2 – the tolerable calculated resistance to bending.

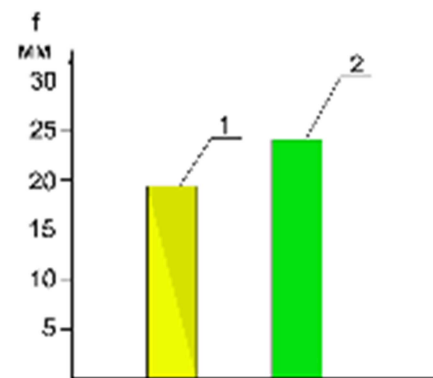


Figure 6. Comparison of a rated deflection with the allowed beam deflection: 1 – rated deflection of a beam; 2 – tolerable calculated deflection of a beam.

Comparison of a deflection of a beam and the allowed deflection is given in figure 6. The analysis shows that deflection size in a beam, arising at a bend under loading, is

80% of size of the allowed deflection. The deflection doesn't exceed the most admissible deflection.

3.3. Tangential Stress

Tangential stresses occur in the wall of the beam. The greatest value they arise in the supporting zones, where the transverse load reaches a maximum value. In connection with this, the beam shall be checked for shearing tangential stresses in the supporting zone.

Beam resistance to splitting is calculated by formula:

$$\tau = \frac{Q \cdot S}{I \cdot b_w} \leq R_{90,d} \quad (8)$$

where Q - calculated transverse force, kN; S - static moment of moving part of cross-section, m^3 ; I - moment of inertia of

cross-section, m^4 ; b_w - wall thickness, m.

Tolerable calculated resistance to bending is determined by formula:

$$R_{90,d} = R_{90,d}^A \cdot m_1 \cdot \prod m_i \quad (9)$$

where $R_{90,d}^A$ - calculated resistance of wood to splitting along wood fibers, MPa (table 1); m_1 - long-term resistance index (table 2); $\prod m_i$ - product of indices of operating conditions, m_b , m_c , m_t (table 2).

Results of calculation of tangent tension and the allowed settlement resistance are given in table 6 and 7.

Table 6. Tangent tension in a beam.

Parameters	q^d , kN/m	l , m	Q , kN	S , m^4	I , m^4	b_w , m	Results, MPa
Tangent tension in a beam	1.5	6.0	4.5	$4.9 \cdot 10^{-3}$	$99.2 \cdot 10^{-6}$	0.043	0.05

Table 7. Tolerable calculated tangent tension in a beam.

Parameters	$R_{90,d}^A$, MPa	m_1	m_b	m_c	m_t	Results, MPa
Tolerable calculated tangent tension in a beam	2.4	0.66	1.0	1.0	1.0	1.58

Comparison of size of tangent tension and the tolerable calculated tangent tension in a beam is given in figure 7. The analysis shows that the size of the tangent tension arising at a bend under loading is only 3% of size of the allowed tolerable calculated tangent tension in a beam.

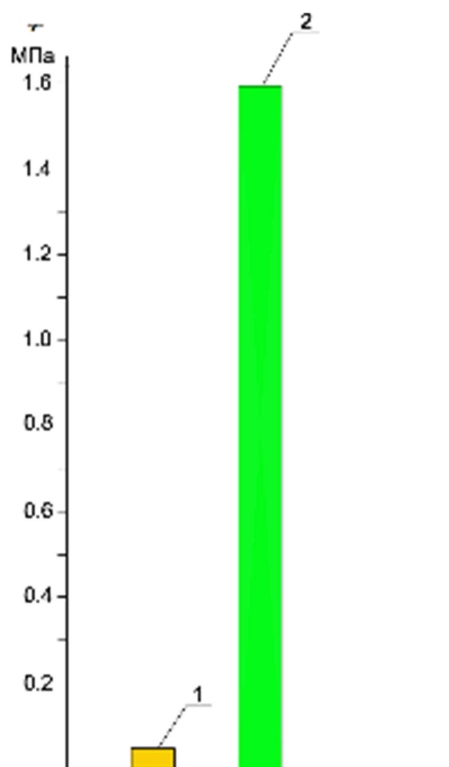


Figure 7. Comparison of tangent tension with the tolerable calculated tangent tension in a beam: 1 – tangent tension in a beam at a bend; 2 – the tolerable calculated tangent tension in a beam.

4. Conclusions

Analyzing the received results it is possible to draw the following conclusions:

1. It is possible to apply the double-T section beams received from round forest products with a core decay as the bearing building constructions.
2. Settlement characteristics of the beams which are stuck together from angular elements correspond to operational loadings.
3. The actual tension in beams received as a result of calculations 1.25-2.5 times less allowed.
4. It is possible to use results of researches when developing the specifications and technical documentation of requirements to the bearing building constructions.
3. Use of round forest products with core decay expands wood resources for construction.

However, the method of calculations accepted in work doesn't consider anisotropic properties of wood of a beam. More exact decision on strength characteristics of I-beams from angular elements requires carrying out further researches and including tests of beams.

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